

THP CANCER CARE EQUIPMENT

ENTUITIVE

DRAWING LIST	
DRAWING No.	DRAWING TITLE
S000	COVER SHEET
S001	GENERAL NOTES
S010	TYPICAL DETAILS
S011	TYPICAL DETAILS
S012	TYPICAL DETAILS
S200	OVERALL FRAMING PLAN - LEVEL 1
S201	ENLARGED FRAMING PLAN - LEVEL 1 NORTH
S202	ENLARGED FRAMING PLAN - LEVEL 1 EAST
S400	FOUNDATION SECTIONS & DETAILS
Grand Total: 9	

3D VIEWS ARE PROVIDED TO AID CLARITY AND MAY NOT BE COMPLETE. REFER TO PLANS, SECTIONS AND SPECIFICATIONS

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9	ISSUED FOR TENDER	2025/12/16
8	ISSUED FOR PERMIT	2025/11/28
7	MOH 2.3 RE-SUBMISSION	2025/06/13
6	MOH 2.3 SUBMISSION	2024/10/11
5	ISSUED FOR 100% CONSTRUCTION DOCUMENTS	2024/09/13
4	MOH 2.3 COSTING SUBMISSION	2024/06/17
3	ISSUED FOR PROGRESS	2024/06/04
2	ISSUED FOR PROGRESS	2024/05/03
1	MOH SUBMISSION	2023/10/18
NO	DESCRIPTION	DATE
SHEET REVISION		
PROJECT: THP CANCER CARE EQUIPMENT 2200 Eglinton Ave W, Mississauga, ON L5M 2N1		
TITLE: COVER SHEET		
PROJECT NO: EN023-01052	DRAWING NO:	
CHECKED: BW	S000	

A. GENERAL

1. WHERE DOCUMENTS ARE REFERENCED IN THE GENERAL AND DESIGN NOTES, THEY SHALL BE THE LATEST EDITIONS OR REVISION, UNLESS NOTED OTHERWISE.
2. READ STRUCTURAL DOCUMENTS IN CONJUNCTION WITH EXISTING STRUCTURAL DRAWINGS, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND OTHER CONTRACT DOCUMENTS.
 1. A COPY OF THE EXISTING STRUCTURAL DRAWINGS BY HALSALL ASSOCIATES LIMITED DATED 2003/01/27 IS AVAILABLE FROM THE CONSULTANT. THIS INFORMATION IS GIVEN SOLELY AS A GUIDE. NO RESPONSIBILITY IS ACCEPTED BY THE OWNER OR THE CONSULTANT FOR ITS CORRECTNESS, NOR SHALL ITS ACCURACY OR ANY OMISSIONS AFFECT THE PROVISION OF THIS CONTRACT.
3. BEFORE PROCEEDING WITH THE WORK, CHECK ALL DIMENSIONS SHOWN ON THE STRUCTURAL DOCUMENTS WITH SITE CONDITIONS AND THOSE SHOWN ON ARCHITECTURAL, MECHANICAL AND ELECTRICAL DOCUMENTS AND REPORT DISCREPANCIES TO THE CONSULTANT.
4. PROVIDE LABOUR, MATERIALS, PLANT AND EQUIPMENT TO COMPLETE ALL STRUCTURAL WORK INDICATED ON THE CONTRACT DOCUMENTS.
5. CARRY OUT CONSTRUCTION OPERATIONS, INCLUDING THE INSTALLATION OF TEMPORARY GUYING AND SHORING REQUIRED, ENSURING THAT THE EXISTING STRUCTURE OR MEMBERS ALREADY ERECTED ARE NOT LOADED IN EXCESS OF THEIR SAFE LOAD CARRYING CAPACITY.

A. REFERENCE STANDARDS/CODES AND ACTS

- 1 CONFORM TO THE REQUIREMENTS OF THE ONTARIO BUILDING CODE 2012 (OBC), AND ANY APPLICABLE ACTS OF ANY AUTHORITY HAVING JURISDICTION, AND THE FOLLOWING:

1. CAN/CSA A23.1 CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION.
 2. CAN/CSA A23.2 METHODS OF TEST FOR CONCRETE.
 3. CAN/CSA A23.3 DESIGN OF CONCRETE STRUCTURES.
 4. CAN/CSA-S16 DESIGN OF STEEL STRUCTURES.
 5. RSIC 1996 REINFORCING STEEL INSTITUTE OF CANADA (RSIC), MANUAL OF STANDARD PRACTICE.
 6. CAN/CSA G40.20/G40.21 STRUCTURAL QUALITY STEEL.
2. ALL STANDARDS AND PUBLICATIONS REFERENCED BY THE STANDARDS NOTED ABOVE ARE TO APPLY.
3. WHERE THERE ARE DIFFERENCES BETWEEN THE DOCUMENTS AND THE STANDARDS, CODES AND ACTS, THE MOST STRINGENT SHALL GOVERN.

C. QUALIFICATIONS

- 1 ANY ORGANIZATION UNDERTAKING TO WELD UNDER THIS CONTRACT SHALL BE CERTIFIED BY THE CANADIAN WELDING BUREAU UNDER REQUIREMENTS OF DIVISION 1 OR DIVISION 2.1 OF W47.1.

D. SUBMITTALS

1. SHOP DRAWINGS
 1. SUBMIT FOR REVIEW BY THE CONSULTANT, DETAILED SHOP DRAWINGS FOR ALL TEMPORARY AND PERMANENT STRUCTURAL WORK INCLUDING, BUT NOT LIMITED TO: CONCRETE FORMWORK; REINFORCING STEEL; STRUCTURAL STEEL, INCLUDING TEMPORARY SHORING; LIGHTWEIGHT STEEL.
 2. THE SCALE OF THE DRAWINGS SHALL BE SUCH THAT THE DETAILS OF THE STRUCTURAL WORK ARE CLEARLY SHOWN, AND IN NO CASE SMALLER THAN 1:50.
 3. EACH DRAWING SUBMITTED FOR CONCRETE FORMWORK; STRUCTURAL STEEL INCLUDING TEMPORARY SHORING, SHALL BEAR THE SEAL AND SIGNATURE OF A QUALIFIED PROFESSIONAL ENGINEER LICENSED BY THE AUTHORITY HAVING JURISDICTION.
 4. CONTRACTOR SHALL ALLOW FOR A 5 WORKING DAY TURN AROUND TIME FOR STRUCTURAL CONSULTANT TO REVIEW THE SHOP DRAWINGS.
 5. CALCULATIONS: SUBMIT CALCULATIONS, BEARING THE SEAL AND SIGNATURE OF PROFESSIONAL ENGINEER LICENSED BY THE AUTHORITY HAVING JURISDICTION, FOR STRUCTURAL WORK, IF REQUESTED BY THE CONSULTANT.
2. MILL TEST REPORTS: MAKE AVAILABLE TO THE CONSULTANT COPIES OF ALL MILL TEST REPORTS COVERING CHEMICAL AND PHYSICAL PROPERTIES OF MATERIALS USED.
 3. CONCRETE MIX DESIGNS: SUBMIT ALL CONCRETE MIX DESIGNS FOR REVIEW. DESCRIBE IN DETAIL ON THE MIX DESIGN SUMMARY THE LOCATION(S) WHERE EACH MIX IS TO BE PLACED IN THE STRUCTURE.
 4. AS-BUILT DRAWINGS: MARK ON A COMPLETE SET OF REPRODUCIBLE AS-BUILT DRAWINGS ANY CHANGES, ADDITIONS, OR DELETIONS THAT OCCUR DURING CONSTRUCTION AS A RESULT OF THE CONTRACTOR'S WORK, CHANGE OF ORDERS OR FOR ANY OTHER REASON.
5. SUBMIT UNDERPINNING PROCEDURES AND DETAILS.

E. MATERIALS

- PROVIDE ONLY NEW STRUCTURAL MATERIALS IN ACCORDANCE WITH THE REFERENCE STANDARDS AND THE FOLLOWING, UNLESS OTHERWISE NOTED.
1. CONCRETE: CONFORM TO THE REQUIREMENTS OF CSA-A23.1 AND THE FOLLOWING:
 1. EXPOSED TO FREEZE-THAW AND CHLORIDES [EXPOSURE CLASS C-1]: $f_c = 35 \text{ MPa}$.
 2. EXPOSED TO FREEZE-THAW [EXPOSURE CLASS F-1]: $f_c = 30 \text{ MPa}$.
 3. NOT EXPOSED: $f_c = 25 \text{ MPa}$.
 2. REINFORCEMENT: CONFORM TO CSA G30 SERIES, $F_y = 400 \text{ MPa}$ FOR ALL REINFORCEMENT. ALL REINFORCEMENT IS TO BE BLACK EXCEPT WHERE THE SUFFIX C IS USED TO DESIGNATE EPOXY COATED REINFORCEMENT.
 3. WELDED WIRE FABRIC: CONFORM TO CSA G30 SERIES, GRADE 386, IN FLAT SHEETS.
 4. STRUCTURAL STEEL:
 1. STRUCTURAL WIDE FLANGE AND WELDED WIDE FLANGE SHAPES (W, WWF) TO CONFORM TO CAN/CSA-G40.20/G40.21 GRADE 350W.
 2. ANGLES AND CHANNELS (L, C) TO CONFORM TO CAN/CSA-G40.20/G40.21 GRADE 300W.
 3. HOLLOW STRUCTURAL SECTIONS (HSS) TO CONFORM TO ASTM A500 GRADE C.
 5. HOT DIP GALVANIZING: CONFORM TO CSA G164, MINIMUM ZINC COATING OF 600 g/m^2 .
 6. STRUCTURAL BOLTS SHALL CONFORM TO ASTM F3125 (GRADES A325, F1852, A490 AND F2280), NUTS SHALL CONFORM TO ASTM A563, WASHERS SHALL CONFORM TO ASTM F436.
 7. WELDED STUD SHEAR CONNECTORS: HEADED STUDS SHALL BE MANUFACTURED BY NELSON (OR APPROVED EQUIVALENT) AND SHALL BE MADE FROM ASTM A-108 COLD ROLLED, DEFORMED WIRE MEETING THE MECHANICAL PROPERTIES OF ASTM A-496 AND SHALL BE WELDED PER THE MANUFACTURER'S RECOMMENDATIONS. STUDS SHALL BE 19 mm IN DIAMETER AND SHALL HAVE A LENGTH (AFTER WELDING) OF 75 mm WHEN 38 mm DECK IS SPECIFIED AND 115 mm WHEN 76 mm DECK IS SPECIFIED.
 8. ANCHOR BOLTS: GRADE A307 OR 300W THREADED ROD CONFORMING TO CSA G40.21-M.
 9. CONCRETE ANCHORS: HEADED STUDS FROM ASTM A-108 COLD ROLLED DEFORMED WIRE MEETING ASTM A-496.
 10. STEEL DECK: CONFORM TO ASTM A653M GRADE A OR B, MINIMUM STEEL CORE THICKNESS OF 0.76 mm. ACTUAL STEEL CORE THICKNESS IS TO BE DETERMINED BY THE SUPPLIER'S ENGINEER AND SHALL SATISFY ALL REQUIRED DESIGN CRITERIA. PROTECTIVE COATING - WIPE COATED STEEL DESIGNATION ZF075.
 11. RIGID INSULATION (FOR USE WITH FOUNDATIONS AND THE LIKE): EXTRUDED POLYSTYRENE WITH A MINIMUM COMPRESSIVE STRENGTH OF 0.24 MPa UNLESS NOTED OTHERWISE.

F. EXECUTION

1. FOUNDATIONS
1. A COPY OF THE SOIL INVESTIGATION REPORT BY PETO MACCALLUM LIMITED DATED 2001/08/24 IS AVAILABLE FROM THE CONSULTANT. READ THIS REPORT, VISIT THE SITE AND THOROUGHLY FAMILIARIZE YOURSELF WITH ALL SURFACE AND SUBSURFACE CONDITIONS. THIS INFORMATION IS GIVEN SOLELY AS A GUIDE. NO RESPONSIBILITY IS ACCEPTED BY THE OWNER OR THE CONSULTANT FOR ITS CORRECTNESS, NOR SHALL ITS ACCURACY OR ANY OMISSIONS AFFECT THE PROVISION OF THIS CONTRACT.
2. FOUND ALL FOOTINGS (AND UNDERPINNING) ON SOIL CAPABLE OF SUSTAINING AN UNFACTORED BEARING STRESS OF 400 kN/m².
3. FOUND ALL FOOTINGS WHICH WILL BE EXPOSED TO FROST ACTION IN THE COMPLETED BUILDING A MINIMUM OF 1200 mm BELOW FINISHED GRADE.
4. DO NOT EXCEED A RISE OF 7 IN A RUN OF 10 IN THE LINE OF SLOPE BETWEEN ADJACENT FOOTING EXCAVATIONS OR ALONG STEPPED FOOTINGS. FOR STEPPED FOOTINGS, USE STEPS NOT EXCEEDING 600 mm IN HEIGHT AND 1200 mm (MIN.) IN LENGTH.
5. SOIL BEARING CAPACITY SPECIFIED MUST BE VERIFIED IN WRITING BY THE SOIL ENGINEER PRIOR TO THE PLACING OF FOOTINGS AND ANY NON-CONFORMANCE WITH THE SPECIFIED MINIMUM CAPACITIES MUST BE IMMEDIATELY REPORTED TO THE STRUCTURAL ENGINEERS.
6. BELOW SLABS ON GRADE BACKFILL USING NATIVE MATERIALS OR ENGINEERED FILL APPROVED BY THE GEOTECHNICAL CONSULTANT AND COMPACT IN MAX 150 mm LIFTS TO 98% SPMD.
7. PROVIDE TEMPORARY FROST PROTECTION, DURING CONSTRUCTION, FOR ALL FOUNDATIONS WHICH ARE NOT FOUND A MINIMUM OF 1200 mm BELOW GRADE.
8. FOUND NEW FOOTINGS WHICH ARE LOCATED ADJACENT TO EXISTING FOOTINGS, AT THE SAME ELEVATION AS THE EXISTING FOOTINGS, UNLESS NOTED OTHERWISE.
9. INSULATION IS SHOWN WHERE REQUIRED FOR PROTECTION OF THE FOUNDATIONS FROM DAMAGE DUE TO FROST ACTION ONLY. REFER TO ARCHITECTURAL DRAWINGS FOR FOUNDATION INSULATION NOT SHOWN ON THE STRUCTURAL DRAWINGS.
10. DO NOT COMPACT CLOSER THAN 1800 mm FROM WALLS WITH HEAVY EQUIPMENT. USE LIGHT HAND CONTROLLED EQUIPMENT WITHIN 1800 mm FROM WALLS.

2. SLAB-ON-GRADE
 1. THE EXISTING SLAB-ON-GRADE HAS BEEN ASSUMED TO SUSTAIN A MINIMUM SLS BEARING PRESSURE OF 25 kPa WITHOUT SETTLEMENT

3 CONCRETE

1. THE CONTRACTOR SHALL ENSURE THAT REINFORCING STEEL IS ADEQUATELY BRACED AGAINST MOVEMENT DURING CONCRETE PLACING.
2. FABRICATE REINFORCEMENT IN ACCORDANCE WITH CAN/CSA A23.1 AND THE RSIC MANUAL OF STANDARD PRACTICE.
3. PERFORM FORMING OPERATION AND PLACE HARDWARE SO THAT FINISHED CONCRETE WILL BE WITHIN THE TOLERANCES SET OUT IN CAN/CSA A23.1.
4. FOLLOW MANUFACTURER'S INSTRUCTIONS REGARDING INSTALLATION PROCEDURES AND MINIMUM EMBEDMENT OF ANCHORS.
5. GROUT BENEATH PLATES BEARING ON CONCRETE WITH AN APPROVED NON-SHRINK FLOWABLE GROUT. CONFORM TO THE MANUFACTURER'S DIRECTIONS FOR MIXING AND PLACING GROUT. COMPLETELY FILL VOIDS BENEATH STEEL BASES ON CONCRETE WITH AN APPROVED NON-SHRINK 35 MPa GROUT.
6. REINFORCEMENT IDENTIFIED AS 'CONTINUOUS' SHALL TERMINATE WITH STANDARD END HOOKS AND SHALL BE LAPPED WITH CLASS 'B' TENSION LAP SPICES.
7. REINFORCEMENT LENGTHS NOTED IN TYPICAL DETAILS ARE MINIMUM LENGTHS UNLESS NOTED OTHERWISE.
8. OPENINGS, SLEEVES, EMBEDDED DUCTS:
 1. NO OPENINGS SHALL BE MADE IN EXISTING PRECAST SLAB OR FOUNDATION WALLS UNLESS REVIEWED AND APPROVED BY THE CONSULTANT
9. LAP SPICES FOR WELDED WIRE FABRIC (WWF) SHALL BE:
 1. 152X152 WWF 500 mm
 2. 102X102 WWF 350 mm
 3. 51X51 WWF 250 mm
10. CONCRETE COVER:
 1. COVER SHALL BE MEASURED FROM THE DEEPEST POINT TEXTURED CONCRETE SURFACE (OR REGLET/REVEAL) TO THE NEAREST DEFORMATION OF REINFORCEMENT. REINFORCEMENT INCLUDES TIES / STIRRUPS AND MAIN REINFORCEMENT.
 2. ALL CONCRETE CAST AGAINST EARTH IS TO HAVE 75 mm cover, UNLESS NOTED OTHERWISE.
 3. ALL CONCRETE EXPOSED TO EARTH, INCLUDING CONCRETE CAST AGAINST FORMS AND SUBSEQUENTLY EXPOSED TO EARTH, IS TO HAVE 50 mm COVER, UNLESS NOTED OTHERWISE.
11. WHERE REINFORCEMENT IS NOT SPECIFICALLY IDENTIFIED ON THE DRAWINGS, PROVIDE 152 x 152 MM#18.7 x MM#18.7 WELDED WIRE FABRIC AT IN SLABS ON GRADE, OR WALKS AND 51 x 51 MM#5.6 x MM#5.6 TOPPINGS 60 mm IN THICKNESS OR GREATER.
12. PLACING CONCRETE

1. CONFORM TO REQUIREMENTS OF CSA A23.1. AND THE FOLLOWING:
 1. IMMEDIATELY BEFORE PLACING CONCRETE, CLEAN FORMS AND REINFORCEMENT OF FOREIGN MATTER.
 2. DO NOT USE CONCRETE MIXED MORE THAN TWO HOURS AFTER INTRODUCTION OF MIXING WATER.
 3. DURING HOT WEATHER CONDITIONS, DO NOT USE CONCRETE MIXED MORE THAN ONE HOUR AFTER INTRODUCTION OF MIXING WATER.
 4. ALLOW 24 HOURS MINIMUM AFTER PLACING CONCRETE IN COLUMNS, PIERS, OR WALLS BEFORE PLACING CONCRETE IN BEAMS OR SLABS SUPPORTED THEREON.
2. PLACE CONCRETE ON AND STEEL DECK FLOORS IN A MANNER THAT AVOIDS PILING UP OF CONCRETE. DO NOT DROP CONCRETE DIRECTLY FROM BUCKETS, BUT EMPLOY SUITABLE MEANS OF DISTRIBUTION. WET DOWN DECK DURING HOT WEATHER PRIOR TO CONCRETING.
 1. REMOVE CONCRETE SPILLED ONTO FORMS AROUND HOISTING EQUIPMENT BEFORE DEPOSITING CONCRETE IN THESE AREAS.
13. CURING CONCRETE.
 1. CURE ALL CONCRETE IN ACCORDANCE WITH CSA A23.1, THE CONCRETE SUPPLIERS REQUIREMENTS AND AS SPECIFIED HEREIN.
14. PROTECTION
 1. CONFORM TO THE REQUIREMENTS OF CSA A23.1. PROTECT FRESHLY DEPOSITED CONCRETE FROM FREEZING, PREMATURE DRYING AND EXTREMES OF TEMPERATURE. MAINTAIN CONCRETE WITH MINIMAL MOISTURE LOSS AT A RELATIVELY CONSTANT TEMPERATURE FOR THE PERIOD OF TIME NECESSARY FOR THE HYDRATION OF THE CEMENT AND TO ACHIEVE THE SPECIFIED STRENGTH OF THE CONCRETE.
 2. PROVIDE SUFFICIENT INSULATION, AND HEAT AS NECESSARY, TO PREVENT FREEZING OF FROST SUSCEPTIBLE SOIL WHICH LIES AGAINST STRUCTURAL ELEMENTS; IN PARTICULAR PROTECT SOIL BENEATH FOOTINGS AND BEHIND FOUNDATION WALLS UNTIL THE BUILDING IS COMPLETED.
 3. CRACK REPAIR: PRIOR TO COMPLETION OF THE PROJECT AND IN ANY CASE NOT SOONER THAN 28 DAYS AFTER CONCRETE HAS BEEN PLACED, EXAMINE CONCRETE FLOOR SURFACES AND REPAIR ALL MAJOR CRACKS IN THEM. ROUT CRACKS OUT WITH MECHANICAL ROUTER TO 13 mm (1/2") SQUARE APPROXIMATE CROSS SECTION. THEN CLEAN AND FILL CRACKS IN SAME MANNER AS SAW CUTS IN SLAB-ON-GRADE.
15. THE USE OF SHOTCRETE TO CONSTRUCT ANY PART OF THE WORK SHALL BE AT THE SOLE DISCRETION OF THE CONSULTANT.

- #### 4 POST-INSTALLED ANCHORS

1. EXCEPT WHERE INDICATED ON THE DRAWINGS, POST-INSTALLED ANCHORS SHALL CONSIST OF THE FOLLOWING ANCHOR TYPES AS PROVIDED BY HILTI (CANADA) CORPORATION.
 1. ANCHORAGE TO CONCRETE:
 1. ADHESIVE ANCHORS FOR CONCRETE USE:
 1. HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HIT-Z ROD FOR FAST CURE APPLICATIONS.
 1. HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT SYSTEM FOR FAST CURE APPLICATIONS.
 3. HILTI HIT-RE 500-SD EPOXY ADHESIVE ANCHORING SYSTEM FOR SLOW CURE APPLICATIONS.
 4. HILTI HIT-RE 500 EPOXY ADHESIVE ANCHORING SYSTEM FOR SLOW CURE APPLICATIONS.
 5. STEEL ANCHOR ELEMENT SHALL BE HILTI HIS-N INTERNALLY THREADED INSERTS, HILTI HAS-E CONTINUOUSLY THREADED ROD, OR CONTINUOUSLY DEFORMED STEEL REBAR.
 2. REBAR DOWELING INTO CONCRETE:
 1. ADHESIVE ANCHORS FOR CRACKED AND UNCRACKED CONCRETE USE:
 1. HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT SYSTEM WITH CONTINUOUSLY DEFORMED REBAR.
 2. HILTI HIT-RE 500-SD EPOXY ADHESIVE ANCHORING SYSTEM WITH CONTINUOUSLY DEFORMED REBAR.
 3. HILTI HIT-RE 500 EPOXY ADHESIVE ANCHORING SYSTEM WITH CONTINUOUSLY DEFORMED REBAR.

2. ANCHOR CAPACITY USED IN DESIGN HAS BEEN BASED ON THE TECHNICAL DATA PUBLISHED BY HILTI. SUBSTITUTION REQUESTS FOR ALTERNATE ANCHORS MUST BE APPROVED IN WRITING BY THE CONSULTANT PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS DEMONSTRATING THAT THE ALTERNATIVE ANCHOR IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED FOR COMPLIANCE WITH THE RELEVANT BUILDING CODE. ADHESIVE ANCHOR EVALUATION WILL ALSO CONSIDER CREEP, IN-SERVICE TEMPERATURE, AND INSTALLATION TEMPERATURE.

3. INSTALL ANCHORS PER THE MANUFACTURER'S WRITTEN INSTRUCTIONS.

4. THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE CONSULTANT MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.

5. ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN STRICT ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.

6. EXISTING REINFORCEMENT IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT, THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE EXISTING REINFORCEMENT AT THE LOCATIONS OF THE CONCRETE ANCHORS, BY HILTI FERROSCAN, HILTI PS 1000, GPR, X-RAY, CHIPPING OR OTHER MEANS.

- ## 5 ALTERATIONS AND/OR CONNECTIONS TO EXISTING STRUCTURE

1. PROPOSED SCHEDULE OF WORK TO BE COORDINATED WITH ALL SUBTRADES, THE CONSULTANT AND OWNER.
2. PROPOSED SEQUENCE OF WORK TO BE SUBMITTED TO THE CONSULTANT FOR REVIEW PRIOR TO START OF WORK.
3. INSPECT THE EXISTING BUILDING AND BECOME THOROUGHLY FAMILIAR WITH THE EXISTING CONDITIONS.
4. PRIOR TO PROCEEDING WITH THE WORK, DETERMINE THE EXACT FOUNDING ELEVATIONS OF EXISTING FOOTINGS ADJACENT TO THE NEW WORK. REPORT THESE FINDINGS TO THE CONSULTANT.
5. PRIOR TO FABRICATION OF STRUCTURAL STEEL, OPEN UP ALL AREAS WHERE CONNECTIONS ARE TO BE MADE TO EXISTING WORK AND TAKE FIELD MEASUREMENTS. MODIFY METHODS FOR CONNECTING TO SUIT SITE CONDITIONS FOUND AND TO THE APPROVAL OF THE CONSULTANT. CARRY OUT LOCAL REPAIRS TO THE EXISTING WORK AS NECESSARY AND AS DIRECTED BY THE CONSULTANT.
6. SHORE EXISTING WORK AS REQUIRED UNTIL ALL NEW WORK HAS BEEN COMPLETED AND REVIEWED BY THE CONSULTANT.
7. SHORE FLOORS AS REQUIRED TO SUPPORT CRANES, HOISTS AND OTHER CONSTRUCTION EQUIPMENT.
8. DO NOT CUT CONCRETE REINFORCEMENT UNLESS REVIEWED AND APPROVED BY THE CONSULTANT.
9. WHERE REQUIRED TO AVOID CUTTING EXISTING REINFORCEMENT, MODIFY THE LAYOUT OF NEW THROUGH BOLTS, EXPANSION ANCHORS AND OTHER ANCHORING DEVICES.
10. MAKE GOOD THE EXISTING WORK.

6 CUTTING AND CORING OF EXISTING STRUCTURE

1. PRIOR TO CUTTING AND CORING ANY OPENINGS IN THE EXISTING BUILDING, PROVIDE THE CONSULTANT WITH A SLEEVING DRAWING INDICATING THE SIZE AND LOCATION OF OPENING RELATIVE TO BUILDING GRID LINES. EXISTING OPENINGS IN THE VICINITY OF THE NEW OPENING MUST ALSO BE SHOWN.
2. ALL DIMENSIONS PROVIDED TO THE CONSULTANT ARE TO BE CONFIRMED WITH THE APPROPRIATE CONTRACTOR (MECHANICAL OR ELECTRICAL) PRIOR TO CUTTING/CORING.
3. ANY REVISIONS TO THE DIMENSIONS BY THE CONSULTANT MUST BE REVIEWED BY THE APPROPRIATE CONTRACTOR PRIOR TO CUTTING/CORING.
4. EXISTING REINFORCEMENT AND EMBEDDED SERVICES MUST BE LOCATED PRIOR TO CUTTING/CORING. THIS REINFORCEMENT IS TO BE LOCATED BY A POSITIVE MEANS, (I.E. X-RAYING, LOCAL CHIPPING OF SLAB- WHERE PERMITTED BY THE CONSULTANT, USE OF COVER METER).
5. AFTER REINFORCEMENT AND EMBEDDED SERVICES HAS BEEN LOCATED IN THESE AREAS, NOTIFY CONSULTANT WHO WILL REVIEW AND APPROVE LOCATION PRIOR TO CUTTING/CORING. MAKE ANY NECESSARY ADJUSTMENTS TO THE HOLE LOCATION AS DIRECTED BY THE CONSULTANT.
6. FOR ANY OPENINGS WHICH ARE TO BE SAWCUT INTO THE EXISTING STRUCTURE, PRE-DRILL THE CORNERS USING A 100 mm DIAMETER CORE DRILL. DO NOT OVERTCUT CORNERS OF OPENING.
7. ALL PRICES FOR CUTTING/CORING ARE TO INCLUDE ANY COSTS ASSOCIATED WITH X-RAYING, CHIPPING, ETC.
8. FOR ANY AREAS WHERE REINFORCEMENT IS CUT, THE CONTRACTOR IS TO INDICATE THE DIRECTION AND LAYER OF REINFORCEMENT ON THE AS-BUILT SLEEVING DRAWINGS.
9. FOR LARGE OPENINGS THROUGH A FLOOR AREA, ADDITIONAL REINFORCEMENT OF THE SLAB MAY BE REQUIRED. THE CONSULTANT WILL ISSUE ADDITIONAL DETAILS AS REQUIRED.

G. QUALITY CONTROL

- ## 1. GENERAL

- A. IMPLEMENT A SYSTEM OF QUALITY CONTROL TO ENSURE THAT THE MINIMUM STANDARDS SPECIFIED HEREIN ARE ATTAINED.
- B. BRING TO THE ATTENTION OF THE CONSULTANT ANY DEFECTS IN THE WORK OR DEPARTURES FROM THE CONTRACT DOCUMENTS, WHICH MAY OCCUR DURING CONSTRUCTION. THE CONSULTANT WILL DECIDE UPON CORRECTIVE ACTION AND GIVE RECOMMENDATIONS IN WRITING.
- C. THE CONSULTANT'S GENERAL REVIEW DURING CONSTRUCTION AND INSPECTION AND TESTING BY INDEPENDENT INSPECTION AND TESTING AGENCIES REPORTING TO THE CONSULTANT ARE BOTH UNDERTAKEN TO INFORM THE OWNER/CLIENT OF THE CONTRACTOR'S PERFORMANCE AND SHALL IN NO WAY AUGMENT THE CONTRACTOR'S QUALITY CONTROL OR RELIEVE THE CONTRACTOR OF CONTRACTUAL RESPONSIBILITY.

- ## 2. NOTIFICATION

- A. PRIOR TO COMMENCING SIGNIFICANT SEGMENTS OF THE WORK, GIVE THE CONSULTANT AND INDEPENDENT INSPECTION AND TESTING COMPANIES APPROPRIATE NOTIFICATION (MINIMUM 24 HOURS) SO AS TO AFFORD THEM REASONABLE OPPORTUNITY TO REVIEW THE WORK. FAILURE TO MEET THIS REQUIREMENT MAY BE CAUSE FOR THE CONSULTANT TO CLASSIFY THE WORK AS DEFECTIVE.

- ### 3. INSPECTION AND TESTING

- A. THE OWNER WILL APPOINT AN INDEPENDENT INSPECTION AND TESTING COMPANY TO MAKE INSPECTIONS OR PERFORM TESTS AS THE OWNER DIRECTS. THE INDEPENDENT INSPECTION AND TESTING COMPANIES SHALL BE RESPONSIBLE ONLY TO THE OWNER AND SHALL MAKE ONLY SUCH INSPECTIONS OR TESTS AS THE OWNER MAY DIRECT. AUTHORIZED INSPECTION AND TESTING SHALL BE PAID FOR BY THE OWNER.

- #### 4. DEFECTIVE MATERIALS AND WORK

- A. WHERE EVIDENCE EXISTS THAT DEFECTIVE WORK HAS OCCURRED OR THAT WORK HAS BEEN CARRIED OUT INCORPORATING DEFECTIVE MATERIALS, THE CONSULTANT MAY HAVE TESTS, INSPECTIONS OR SURVEYS PERFORMED, ANALYTICAL CALCULATIONS OF STRUCTURAL STRENGTH MADE, AND THE LIKE, IN ORDER TO HELP DETERMINE WHETHER THE WORK MUST BE CORRECTED OR REPLACED. TESTS, INSPECTIONS, SURVEYS, OR CALCULATIONS CARRIED OUT UNDER THESE CIRCUMSTANCES WILL BE MADE AT THE CONTRACTOR'S EXPENSE, REGARDLESS OF THEIR RESULTS, WHICH MAY BE SUCH THAT, IN THE CONSULTANT'S OPINION, THE WORK MAY BE ACCEPTABLE.
- B. ALL TESTING SHALL BE CONDUCTED IN ACCORDANCE WITH THE REQUIREMENTS OF THE BUILDING CODE, EXCEPT WHERE THIS WOULD, IN THE CONSULTANT'S OPINION, CAUSE UNDUE DELAY OR GIVE RESULTS NOT REPRESENTATIVE OF THE REJECTED MATERIAL IN PLACE. IN THIS CASE, THE TESTS SHALL BE CONDUCTED IN ACCORDANCE WITH THE STANDARDS GIVEN BY THE CONSULTANT.
- C. MATERIALS OR WORK, WHICH FAIL TO MEET SPECIFIED REQUIREMENTS, MAY BE REJECTED BY THE CONSULTANT WHENEVER FOUND AT ANY TIME PRIOR TO FINAL ACCEPTANCE OF THE WORK REGARDLESS OF PREVIOUS INSPECTION. IF REJECTED, DEFECTIVE MATERIALS OR WORKMANSHIP SHALL BE PROMPTLY REMOVED AND REPLACED OR REPAIRED TO THE SATISFACTION OF THE CONSULTANT, AT NO EXPENSE TO THE OWNER.

<p>CLIENT:</p>	<p>ENTUITIVE</p> <p>120 Bremner Blvd, 4th Floor Toronto, ON M5J 0A8 Canada +1 416 477 5832</p>
<p>CONSULTANT:</p>	<p>SEAL:</p>
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Country	Year	Sample size	Study design	Reference
Australia	1995	1000	Cross-sectional	1
Belgium	1997	1000	Cross-sectional	2
Canada	1996	1000	Cross-sectional	3
France	1997	1000	Cross-sectional	4
Germany	1997	1000	Cross-sectional	5
Italy	1997	1000	Cross-sectional	6
Japan	1997	1000	Cross-sectional	7
Netherlands	1997	1000	Cross-sectional	8
Norway	1997	1000	Cross-sectional	9
Sweden	1997	1000	Cross-sectional	10
Switzerland	1997	1000	Cross-sectional	11
United Kingdom	1997	1000	Cross-sectional	12
United States	1997	1000	Cross-sectional	13

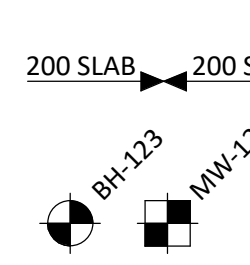

Case no.	Age (years)	Sex	Occupation	Family size
1	25	Male	Student	3
2	25	Male	Student	3
3	25	Male	Student	3
4	25	Male	Student	3
5	25	Male	Student	3
6	25	Male	Student	3
7	25	Male	Student	3
8	25	Male	Student	3
9	25	Male	Student	3
10	25	Male	Student	3
11	25	Male	Student	3
12	25	Male	Student	3
13	25	Male	Student	3
14	25	Male	Student	3
15	25	Male	Student	3

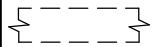



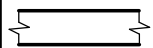



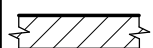





Study	Sample size	Study design	Outcome
1. Kaur et al. (2006)	100	Cross-sectional	Prevalence of 10.0%
2. Kaur et al. (2007)	100	Cross-sectional	Prevalence of 12.0%
3. Kaur et al. (2008)	100	Cross-sectional	Prevalence of 15.0%
4. Kaur et al. (2009)	100	Cross-sectional	Prevalence of 18.0%
5. Kaur et al. (2010)	100	Cross-sectional	Prevalence of 20.0%
6. Kaur et al. (2011)	100	Cross-sectional	Prevalence of 22.0%
7. Kaur et al. (2012)	100	Cross-sectional	Prevalence of 25.0%
8. Kaur et al. (2013)	100	Cross-sectional	Prevalence of 28.0%
9. Kaur et al. (2014)	100	Cross-sectional	Prevalence of 30.0%
10. Kaur et al. (2015)	100	Cross-sectional	Prevalence of 32.0%

9	ISSUED FOR TENDER	2025/12/16
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6	MOH 2.3 SUBMISSION	2024/10/11
5	ISSUED FOR 100% CONSTRUCTION DOCUMENTS	2024/09/13
4	MOH 2.3 COSTING SUBMISSION	2024/06/17
3	ISSUED FOR PROGRESS	2024/06/04
2	ISSUED FOR PROGRESS	2024/05/03
1	MOH SUBMISSION	2023/10/18
NO	DESCRIPTION	DATE
SHEET REVISION		

PROJECT:
THP CANCER CARE EQUIPMENT
2200 Eglinton Ave W,
Mississauga, ON L5M 2N1

TITLE:	
GENERAL NOTES	
PROJECT NO: EN023-01052	DRAWING NO: S001
CHECKED: BW	

ABBREVIATIONS				FORCES	
A, ABV	ABOVE	IT	JOINT	BR	FACTORED BEARING FORCE, kN
A, ROD	ANCHOR ROD			DL	DEAD LOAD
AEC	ARCH EXPOSED CONCRETE	Ød	DEVELOPMENT LENGTH OF REINFORCEMENT		
AESS	ARCH EXPOSED STRUCTURAL STEEL				
AHU	AIR HANDLING UNIT	LG	LONG		
ALT	ALTERNATE	LL	LOWER LAYER	H	FACTORED END SHEAR FORCE ALONG X-X AXIS (Vx), kN
ARCH	ARCHITECTURAL	LLH	LONG LEG HORIZONTAL		
ASL	ACCUMULATED SNOW LOAD	LLV	LONG LEG VERTICAL		
		LO	LOWER OF TWO BEAMS		
B, BOT	BOTTOM	LP	LOW POINT	LL	LIVE LOAD
BL	BOTTOM LOWER LAYER	LSH	LONG SIDE HORIZONTAL		
BM	BEAM	LSV	LONG SIDE VERTICAL	M, Mx	FACTORED BENDING MOMENT ABOUT X-X AXIS, kN-m
BML	BOTTOM MIDDLE LAYER				
BOF	BOTTOM (FOUNDING ELEVATION) OF FOOTING	MAX	MAXIMUM		
BPL	BASE (BEARING) PLATE	MC	MOMENT CONNECTION		
BUL	BOTTOM UPPER LAYER	MECH	MECHANICAL	My	FACTORED BENDING MOMENT ABOUT Y-Y AXIS, kN-m
		MEZZ	MEZZANINE		
C	EPOXY COATED	MIN	MINIMUM		
C/C	CENTRE TO CENTRE	MJ	MOVEMENT JOINT		
C/W	CONNECT WITH	ML	MIDDLE LAYER	P+	FACTORED TENSION AXIAL FORCE, kN
CA, CB	COLUMN ABOVE, BELOW	MOM	MOMENT		
CANT	CANTILEVER	N-S	NORTH-SOUTH	P-	FACTORED COMPRESSION AXIAL FORCE, kN
CJ	CONSTRUCTION JOINT	NC	NOT IN CONTRACT		
CLR	CLEAR	NF	NEAR FACE		
CLS	COMPRESSION LAP SPLICE	No.	NUMBER	P±	FACTORED TENSION / COMPRESSION (REVERSIBLE) FORCE, kN
COL	COLUMN	NTS	NOT TO SCALE		
CONC	CONCRETE				
CONT	CONTINUOUS	OF	OUTSIDE FACE		
COS	CONFIRM ON SITE	OPEN	OPENING	PTF	FACTORED PASS THROUGH FORCE, kN
CP	CAST-IN PLATE	OWSJ	OPEN WEB STEEL JOIST		
CSS	CHANGE IN SPANNING SYSTEM				
		PA, PB	POST ABOVE, BELOW	TM	FACTORED TORSIONAL MOMENT, kN-m
DB	DIVIDER BEAM	PC	PRECAST		
DET	DETAIL	PL	PLATE		
DIA, Ø	DIAMETER	PROJ	PROJECTION	V	FACTORED END SHEAR FORCE ALONG Y-Y AXIS (Vy), kN
DIAG	DIAGONAL				
DIM	DIMENSION	R	RADIUS		
DWG(S)	DRAWING(S)	R/W	REINFORCE WITH	WD	UNFACTORED UNIFORMLY DISTRIBUTED DEAD LOAD ON STEEL DECK, PRECAST OR OWSJ's
DWL(S)	DOWEL(S)	RD	ROOF DRAIN		
		REF	REFERENCE		
E-W	EAST-WEST	REINF	REINFORCE, REINFORCEMENT		
EA	EACH	REQ'D	REQUIRED		
EE	EACH END	REV	REVISION, REVISED		
EF	EACH FACE	RTU	ROOFTOP UNIT	WL	UNFACTORED UNIFORMLY DISTRIBUTED LIVE LOAD ON STEEL DECK, PRECAST OR OWSJ's
EJ, EXP, JT	EXPANSION JOINT				
EL	ELEVATION	SA	SHELF ANGLE		
ELECT	ELECTRICAL	SDF	STEP-DOWN FOOTING		
ELEV	ELEVATOR	SL	SLAB		
EQ	EQUAL	SOG	SLAB-ON-GRADE		
ES	EACH SIDE	SPEC'S	SPECIFICATIONS		
EW	EACH WAY	SQ	SQUARE		
EX, EXIST	EXISTING	STD	STANDARD		
EXT	EXTERIOR	STRUCT	STRUCTURAL		
		SW	SELF WEIGHT		
f _c	CONCRETE COMPRESSIVE STRENGTH	T, THK	THICKNESS	kg	KILOGRAM
FDN	FOUNDATION	T	TOP	m	METRE
FF	FAR FACE	TD	TYPICAL DETAIL	Pa	PASCAL
FIN	FINISHED	TEMP	TEMPERATURE	kPa	KILOPASCAL
FL	FLOOR	TJ	TIE JOIST	kN/m ²	KILONEWTON PER SQUARE METRE (KILOPASCAL)
FMC	FULL MOMENT CONNECTION	TLL	TOP LOWER LAYER		
FTG	FOOTING	TLS	TENSION LAP SPLICE		
f _y	YIELD STRENGTH FOR STEEL REINFORCEMENT	TML	TOP MIDDLE LAYER	MPa	MEGAPASCAL
		TOW	TOP OF WALL		
F _y	YIELD STRENGTH FOR STRUCTURAL STEEL	TUL	TOP UPPER LAYER	N	NEWTON
		TYP	TYPICAL	kN	KILONEWTON
GA	GAUGE	U/N, UNO	UNLESS NOTED OTHERWISE	kN-m	KILONEWTON METRE
GALV	GALVANIZED	U/S	UNDERSIDE		
		UL	UPPER LAYER	kN/m	KILONEWTON PER METRE
H, HORIZ	HORIZONTAL				
HA, HB	HANGER ABOVE, BELOW				
HEF	HORIZONTAL EACH FACE	V, VERT	VERTICAL		
HH	HOOKED EACH END	VBR	VERTICAL BRACING		
HI	HIGHER OF TWO BEAMS	VEF	VERTICAL EACH FACE		
HP	HIGH POINT	VSC	VERTICALLY SLOTTED CONNECTION		
HSC	HORIZONTALLY SLOTTED CONNECTION				
		W/	WITH		
IF	INSIDE FACE	WP	WORK POINT		
INT	INTERIOR	WPA, WPB	WIND POST ABOVE, BELOW		
IR	INTEGRITY REINFORCEMENT	WPL	WALL PLATE		
		WWF	WELDED WIRE FABRIC		
SYMBOLS					
@	AT	200 SLAB	200 SOG	SLAB CHANGE LOCATION	
⊕	CENTRE LINE			BOREHOLE/MONITORING WELL	
⌒	DOUBLE ANGLE				
►	MOMENT CONNECTION				
L	SINGLE ANGLE	CSS	CSS	CHANGE IN SPAN SYSTEM	
▼	VERTICAL CONTROL JOINT IN MASONRY/CONCRETE WALLS				
	BEAM ELEVATION FROM PLAN DATUM				
SDF	STEPPED-DOWN FOOTING				
⬇	WORK POINT				
G1	ABBREVIATIONS AND SYMBOLS				

MARK		INDICATIVE OF
NEW	EXISTING	
		CONCRETE / MASONRY / WOOD STRUCTURE BELOW
		CONCRETE STRUCTURE ABOVE
		CONCRETE UPTURNED BEAMS AND CURBS
		CONCRETE IN SECTION
		MASONRY WALL ABOVE AND IN SECTION
		WOOD SHEAR WALL ABOVE
		WOOD LOAD BEARING WALL ABOVE

NOTES:

1. MAINTAIN MINIMUM SLOPE TO ADJACENT FOOTINGS AND EXCAVATIONS UNLESS NOTED OTHERWISE IN THE GEOTECHNICAL REPORT.
2. LOWER FOOTINGS AS NECESSARY TO ACHIEVE SLOPE NOTED.
3. ENSURE SPECIFIED SLOPE IS NOT COMPROMISED BY DISTURBED SOIL BETWEEN THE EXCAVATIONS.
4. WHERE FOOTINGS ARE STEPPED DOWN, A MAXIMUM LEVEL DIFFERENCE OF 600mm SHOULD BE MAINTAINED.

CF3 REV ADJACENT FOOTINGS AND EXCAVATIONS

FOOTING WITH PIER

ISOMETRIC VIEW

FOOTING WITHOUT PIER

ISOMETRIC VIEW

NOTES:

1. PROVIDE MINIMUM VERTICAL STEEL OF 0.5% OF PIER AREA IN PIER WITH TIES (8-20 MIN) UNLESS NOTED OTHERWISE IN SCHEDULE.
2. PROVIDE DEVELOPMENT AND SPlice LENGTHS AS SHOWN, UNLESS NOTED OTHERWISE.
3. EXTEND DOWELS INTO FOOTING BELOW WHEN HEIGHT OF PIER IS LESS THAN THE COMPRESSION DEVELOPMENT LENGTH OF THE DOWELS BEING DEVELOPED AND WHERE NO PIER EXISTS.
4. ARRANGE VERTICAL BARS AND DETAIL SETS OF TIES AROUND DOWELS IN ACCORDANCE WITH TYPICAL DETAIL CC1.

CF5

FOOTING UNDER CONCRETE COLUMN

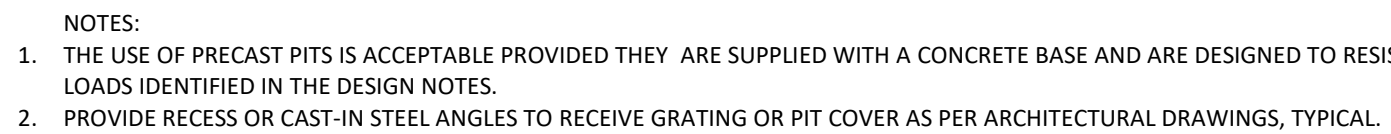
DETAIL NO.	DETAIL TITLE	DRAWING NO.
CONCRETE FOOTINGS		
CF3_en	ADJACENT FOOTINGS AND EXCAVATIONS	S010
CF4	BACKFILL AROUND SERVICES BENEATH STRIP FOOTINGS	S011
CF5	FOOTING UNDER CONCRETE COLUMN	S010
CF6	FOOTING UNDER STEEL COLUMN	S010
CF8	CAISSON SUPPORTING CONCRETE/STEEL COLUMN	S011
CF9	EXCAVATION SUPPORT SYSTEMS & FOUNDATION WALLS	S011
CF11	UNDERPINNING DETAILS	S012
CONCRETE SLABS		
CS11	REINFORCEMENT AT STEPS IN SLAB-ON-GRADE	S012
CS20	DETAILS FOR HOUSEKEEPING PADS	S012
CONCRETE WALLS		
CW1	PITS AND TRENCHES	S011
CW2	REINFORCEMENT DETAILS IN CONCRETE WALLS	S011
GENERAL		
G1_en	ABBREVIATIONS AND SYMBOLS	S010
G2_en	STRUCTURE LEGEND	S010
MASONRY		
M1	LINTELS FOR NON-LOAD BEARING MASONRY WALLS	S012

NOTES:

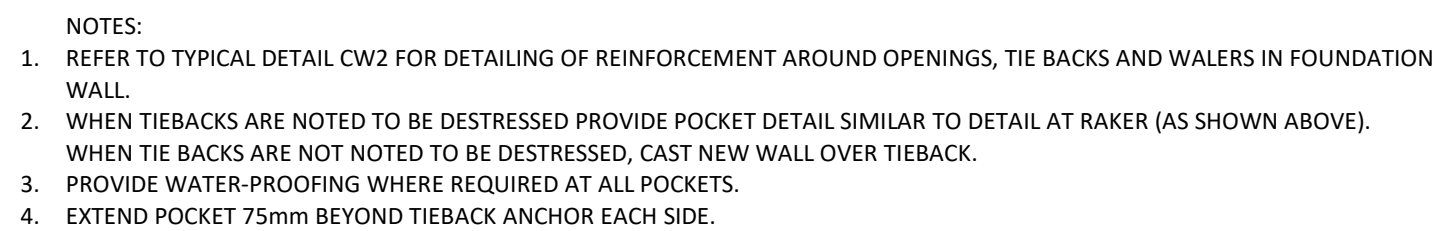
- PROVIDE MINIMUM VERTICAL STEEL OF 0.5% OF PIER AREA IN PIER WITH TIES (8-20 MIN) UNLESS NOTED OTHERWISE IN SCHEDULE.
- PROVIDE DEVELOPMENT AND SPLICE LENGTHS AS SHOWN, UNLESS NOTED OTHERWISE.
- EXTEND DOWELS INTO FOOTING BELOW WHEN HEIGHT OF PIER IS LESS THAN THE COMPRESSION DEVELOPMENT LENGTH OF THE DOWELS BEING DEVELOPED.
- ARRANGE VERTICAL BARS AND DETAIL SETS OF TIES AROUND DOWELS IN ACCORDANCE WITH TYPICAL DETAIL CC1.

CF6
FOOTING UNDER STEEL COLUMN

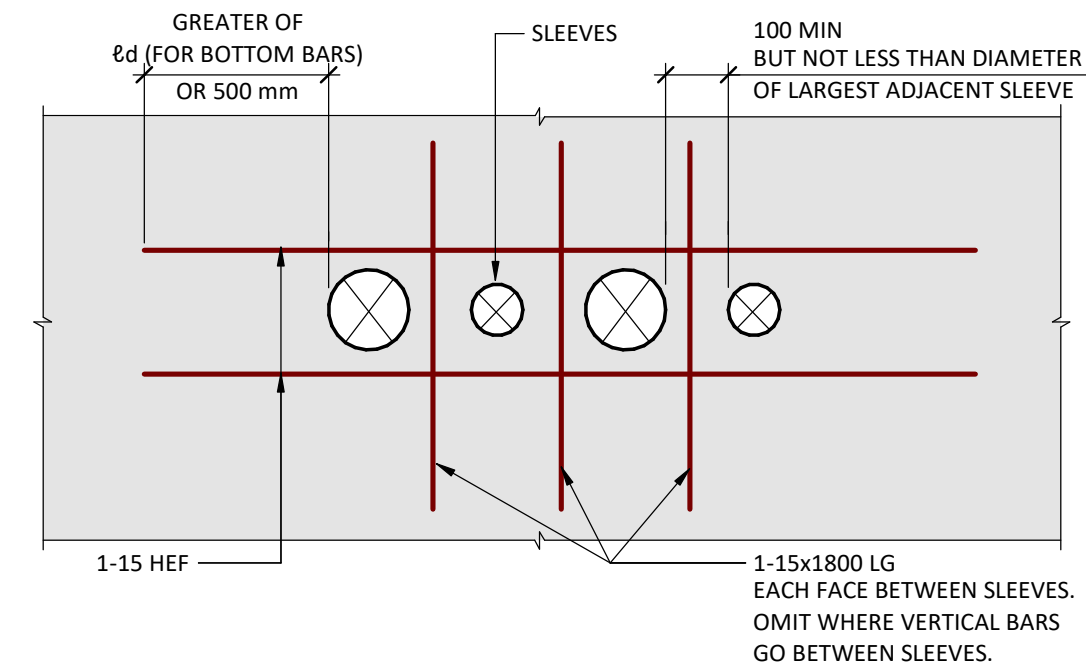
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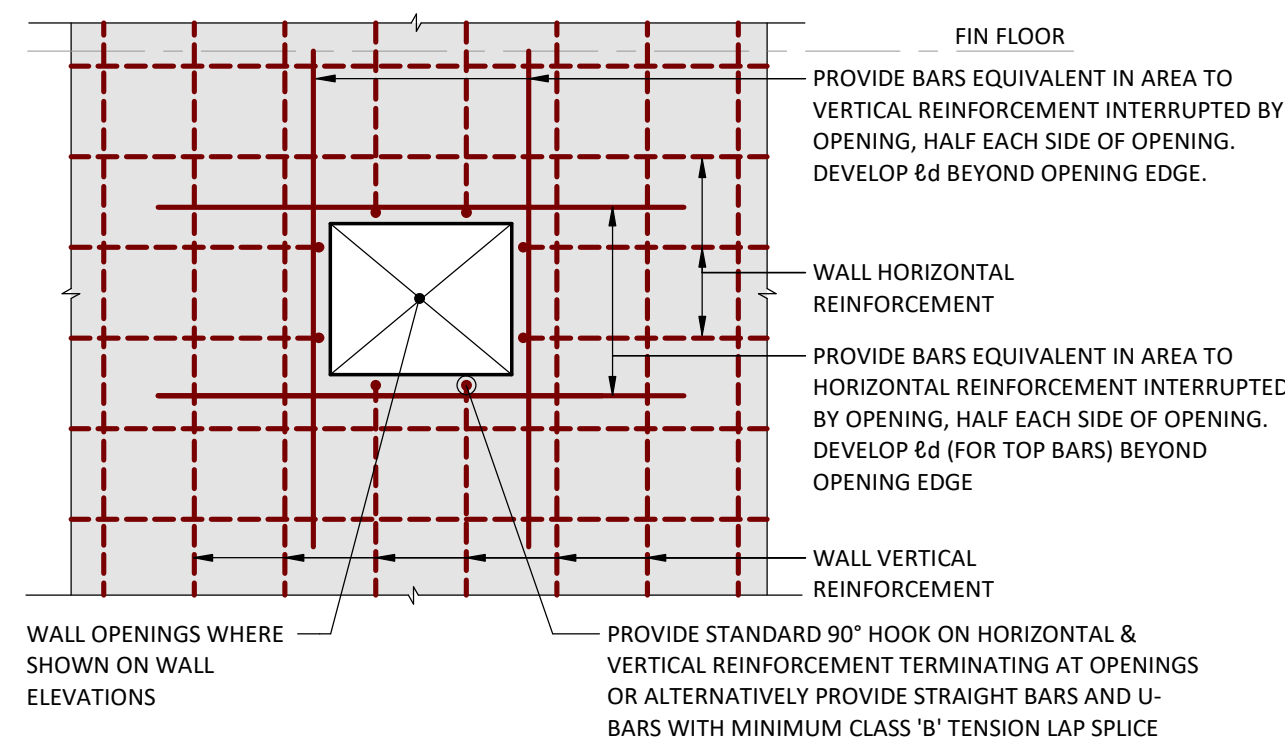
PITS AND TRENCHES



EXCAVATION SUPPORT SYSTEMS & FOUNDATION WALLS

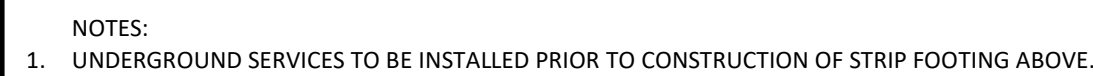


ADDITIONAL REINFORCEMENT FOR SLEEVES, UNLESS NOTED OTHERWISE

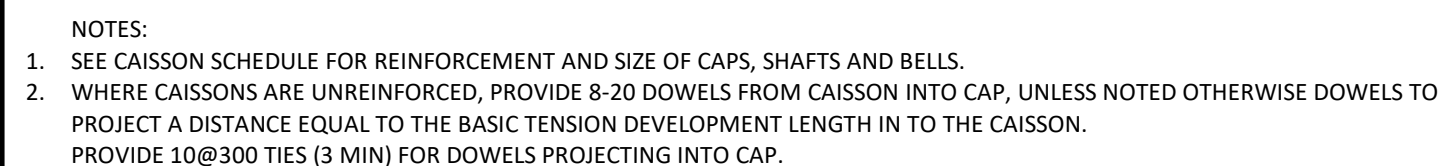


ADDITIONAL REINFORCEMENT FOR OPENINGS, UNLESS NOTED OTHERWISE

REINFORCEMENT DETAILS IN CONCRETE FOUNDATION WALLS



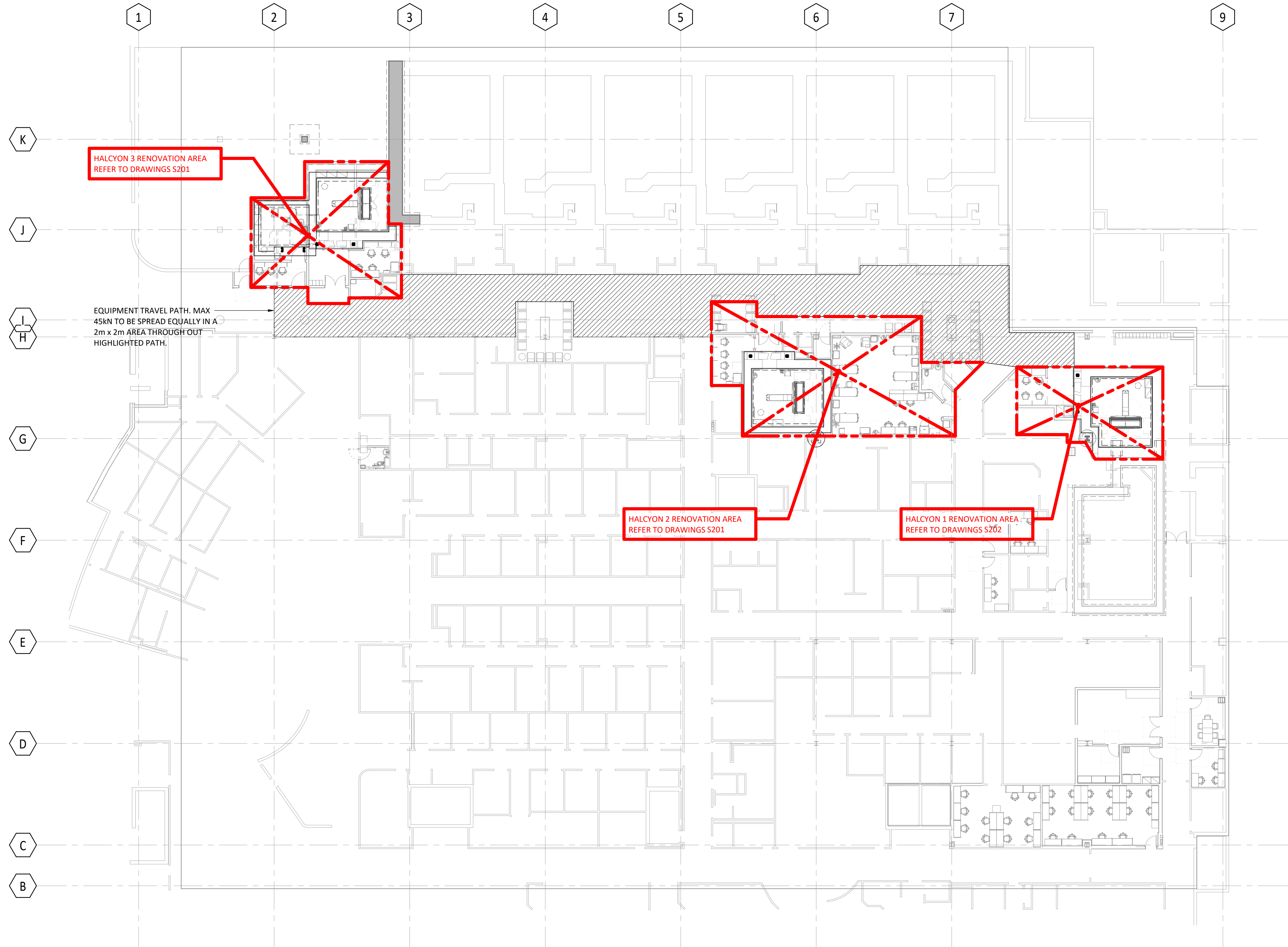
BACKFILL AROUND SERVICES BENEATH STRIP FOOTINGS



CAISSON SUPPORTING CONCRETE/STEEL COLUMN

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S011



OVERALL FRAMING PLAN - LEVEL 1
1 : 200

- 1. FINISHED FLOOR IS AT ELEVATION 161.0 EXCEPT AS CROSSED AND NOTED. ELEVATIONS FOR AREAS CROSSED AND NOTED ARE TO BE READ FROM FINISHED FLOOR ELEVATION.
- 2. EXISTING TOP OF INTERIOR CAISSONS AT ELEVATION 160.0 UNLESS NOTED OTHERWISE AS NOTED IN THE EXISTING STRUCTURAL DRAWINGS.
- 3. EXISTING TOP OF PERIMETER CAISSONS AT ELEVATION 159.65 UNLESS NOTED IN THE EXISTING STRUCTURAL DRAWINGS.
- 4. EXISTING TOP OF COLUMN FOOTINGS AT ELEVATION 160.0 AND WALL FOOTINGS AT ELEVATION 160.8 UNLESS NOTED OTHERWISE.
- 5. ALLOWANCE TO BE PROVIDED TO ENSURE SOIL CONDITIONS BELOW EXISTING FOUNDATIONS ARE NOT DISTURBED DURING CONSTRUCTION.
- 6. TOP OF NEW FOOTING IS AT ELEVATION 160.5 EXCEPT AS CROSSED AND NOTED. ELEVATIONS FOR AREAS CROSSED AND NOTED ARE TO BE READ FROM FINISHED FLOOR ELEVATION.
- 7. PROVIDE PRE-CONDITION ASSESSMENT OF THE EXISTING SOG SURROUNDING THE WORK AREAS. ROUT AND GROUT ANY NEW SOG CRACKS THAT RESULT FROM CONSTRUCTION ACTIVITIES.

CCLIENT:

CCONSULTANT:

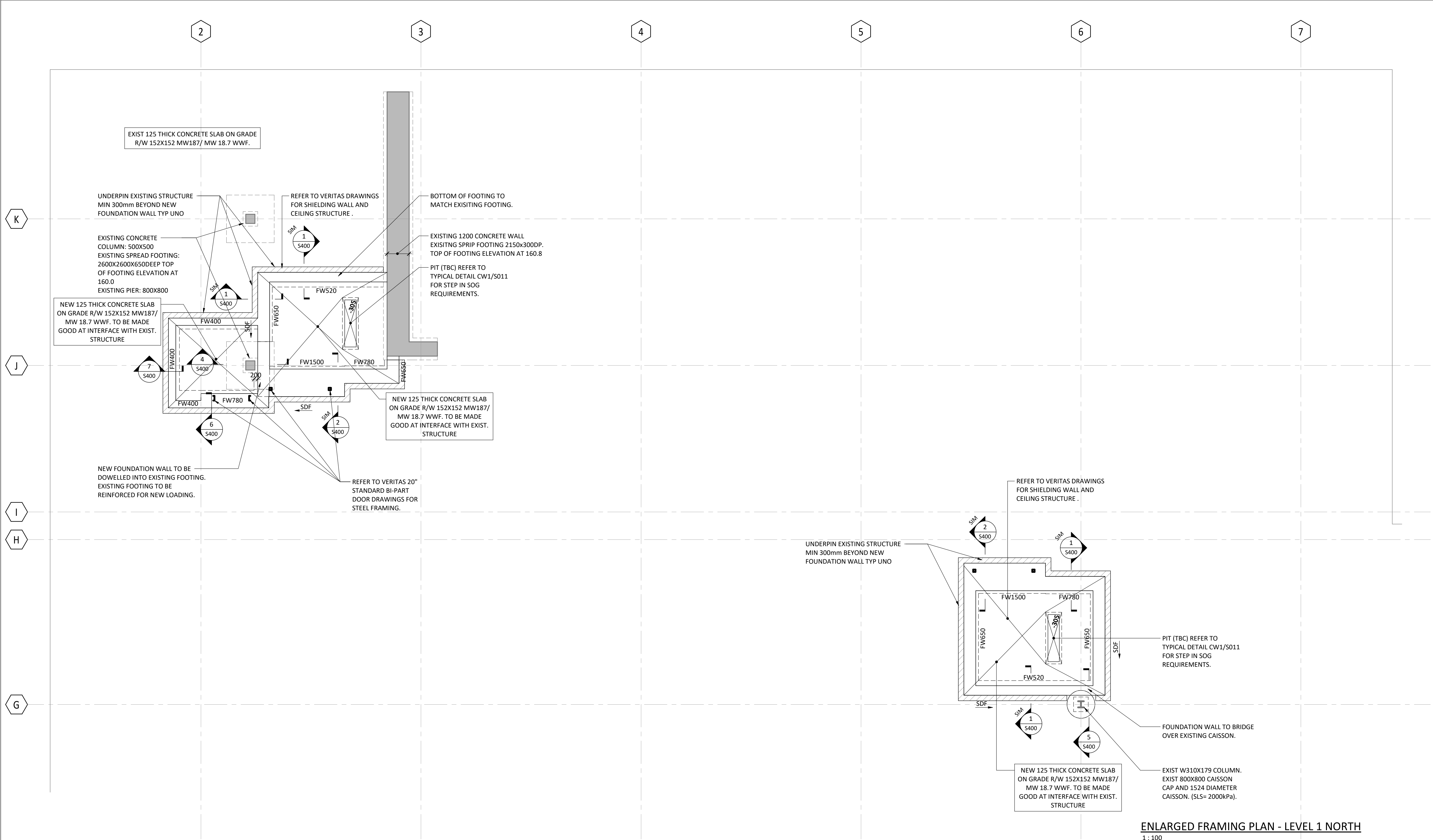
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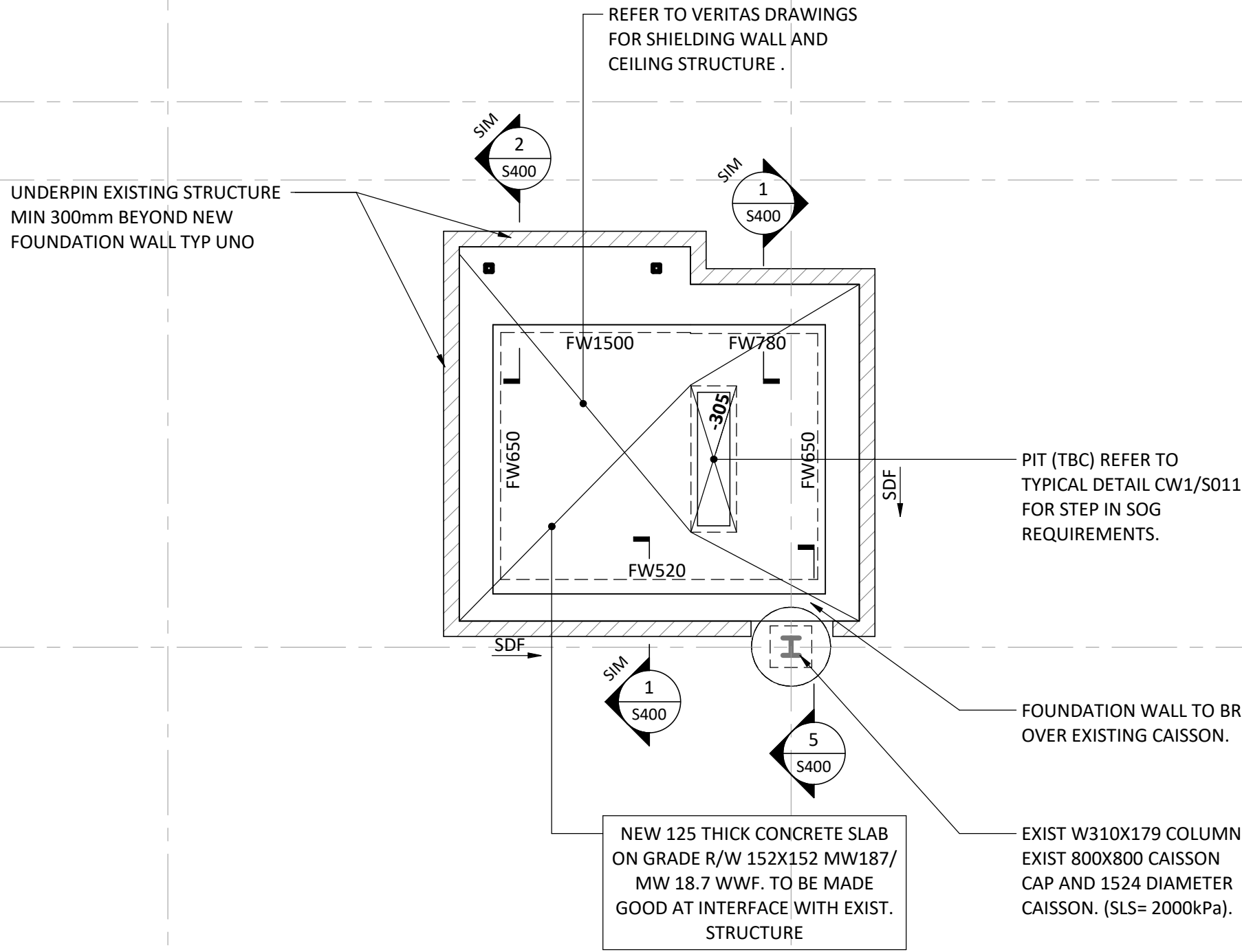
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8	ISSUED FOR PERMIT	2025/11/28
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6	MOH 2.3 SUBMISSION	2024/10/11
5	ISSUED FOR 100% CONSTRUCTION DOCUMENTS	2024/09/13
4	MOH 2.3 COSTING SUBMISSION	2024/06/17
3	ISSUED FOR PROGRESS	2024/06/04
2	ISSUED FOR PROGRESS	2024/05/03
1	MOH SUBMISSION	2023/10/18
NO	DESCRIPTION	DATE
SHEET REVISION		
PROJECT: THP CANCER CARE EQUIPMENT 2200 Eglinton Ave W, Mississauga, ON L5M 2N1		
TITLE: OVERALL FRAMING PLAN - LEVEL 1		
PROJECT NO: EN023-01052	DRAWING NO:	
CHECKED: BW	S200	



TYPE	THICKNESS OF FOUNDATION WALL	FOUNDATION WALL FOOTING		WALL REINFORCEMENT
		WIDTH	DEPTH	
FW400	400	600	300	15@400 HEF & VEF
FW520	520	800	300	15@375 HEF & VEF
FW650	650	800	300	15@300 HEF & VEF
FW780	780	950	300	15@250 HEF & VEF
FW850	850	1000	300	15@225 HEF & VEF
FW910	910	1100	300	15@225 HEF & VEF
FW1500	1500	1650	300	20@200 HEF & VEF



ENLARGED FRAMING PLAN - LEVEL 1 NORTH
1 : 100

- FINISHED FLOOR IS AT ELEVATION 161.0 EXCEPT AS CROSSED AND NOTED. ELEVATIONS FOR AREAS CROSSED AND NOTED ARE TO BE READ FROM FINISHED FLOOR ELEVATION.
- EXISTING TOP OF INTERIOR CAISSONS AT ELEVATION 160.0 UNLESS NOTED OTHERWISE AS NOTED IN THE EXISTING STRUCTURAL DRAWINGS.
- EXISTING TOP OF PERIMETER CAISSONS AT ELEVATION 159.65 UNLESS NOTED IN THE EXISTING STRUCTURAL DRAWINGS.
- EXISTING TOP OF COLUMN FOOTINGS AT ELEVATION 160.0 AND WALL FOOTINGS AT ELEVATION 160.8 UNLESS NOTED OTHERWISE.
- ALLOWANCE TO BE PROVIDED TO ENSURE SOIL CONDITIONS BELOW EXISTING FOUNDATIONS ARE NOT DISTURBED DURING CONSTRUCTION.
- TOP OF NEW FOOTING IS AT ELEVATION 160.5 EXCEPT AS CROSSED AND NOTED. ELEVATIONS FOR AREAS CROSSED AND NOTED ARE TO BE READ FROM FINISHED FLOOR ELEVATION.
- PROVIDE PRE-CONDITION ASSESSMENT OF THE EXISTING SOG SURROUNDING THE WORK AREAS. ROUT AND GROUT ANY NEW SOG CRACKS THAT RESULT FROM CONSTRUCTION ACTIVITIES.

CCLIENT:

CONSULTANT:

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2	ISSUED FOR PROGRESS	2024/05/03
1	MOH SUBMISSION	2023/10/18
NO	DESCRIPTION	DATE

SHEET REVISION

PROJECT:
THP CANCER CARE EQUIPMENT
2200 Eglinton Ave W,
Mississauga, ON L5M 2N1

TITLE:
ENLARGED FRAMING PLAN -
LEVEL 1 NORTH

PROJECT NO:
EN023-01052

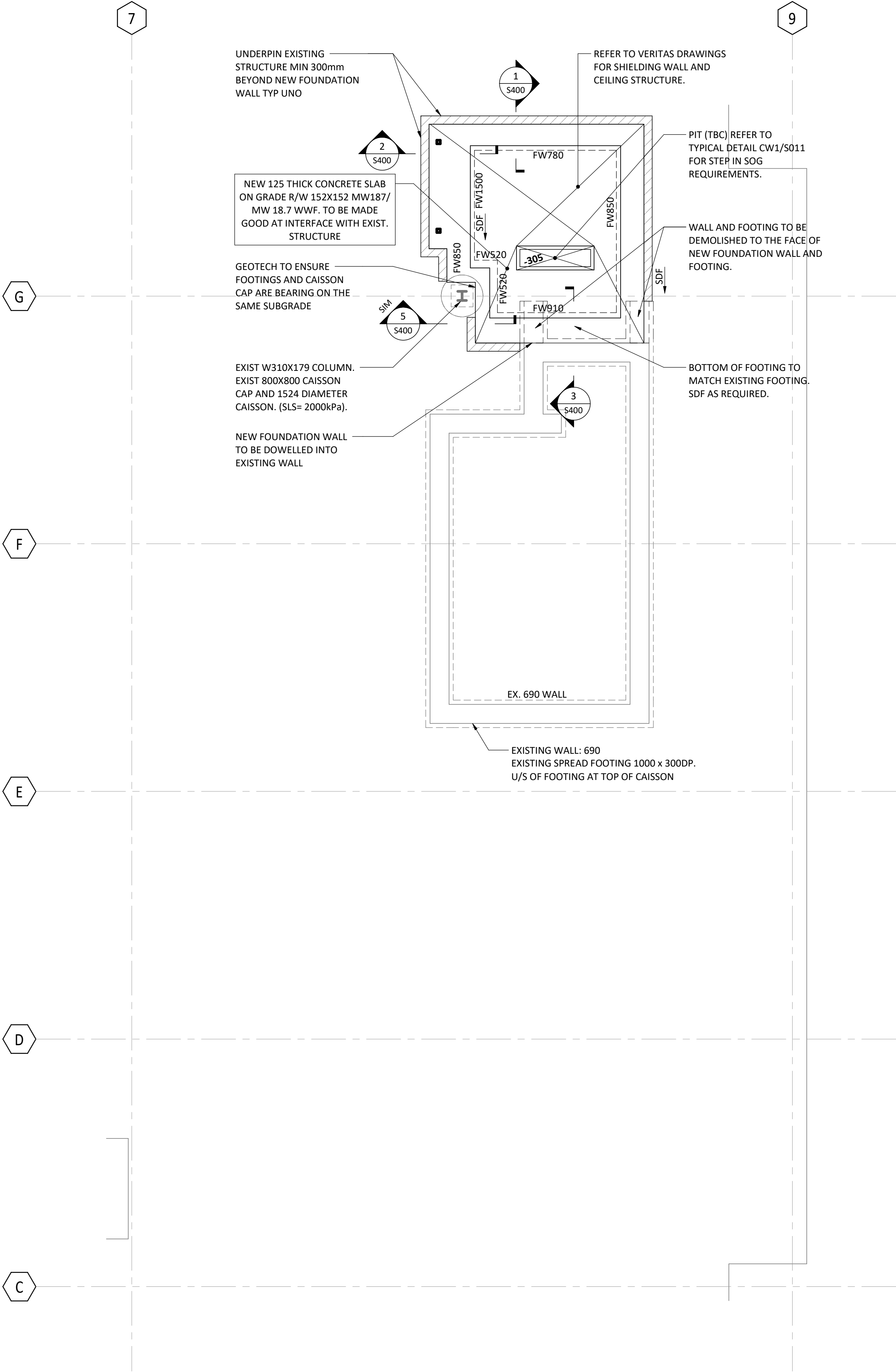
CHECKED:
BW

DRAWING NO:

S201

ENLARGED FRAMING PLAN - LEVEL 1 EAST
1 : 100

1. FINISHED FLOOR IS AT ELEVATION 161.0 EXCEPT AS CROSSED AND NOTED. ELEVATIONS FOR AREAS CROSSED AND NOTED ARE TO BE READ FROM FINISHED FLOOR ELEVATION.
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6. TOP OF NEW FOOTING IS AT ELEVATION 160.5 EXCEPT AS CROSSED AND NOTED. ELEVATIONS FOR AREAS CROSSED AND NOTED ARE TO BE READ FROM FINISHED FLOOR ELEVATION.



TYPE	THICKNESS OF FOUNDATION WALL	FOUNDATION WALL FOOTING		WALL REINFORCEMENT
		WIDTH	DEPTH	
FW400	400	600	300	15@400 HEF & VEF
FW520	520	800	300	15@375 HEF & VEF
FW650	650	800	300	15@300 HEF & VEF
FW780	780	950	300	15@250 HEF & VEF
FW850	850	1000	300	15@225 HEF & VEF
FW910	910	1100	300	15@225 HEF & VEF
FW1500	1500	1650	300	20@200 HEF & VEF

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3	ISSUED FOR PROGRESS	2024/06/04
2	ISSUED FOR PROGRESS	2024/05/03
1	MOH SUBMISSION	2023/10/18

SHEET REVISION

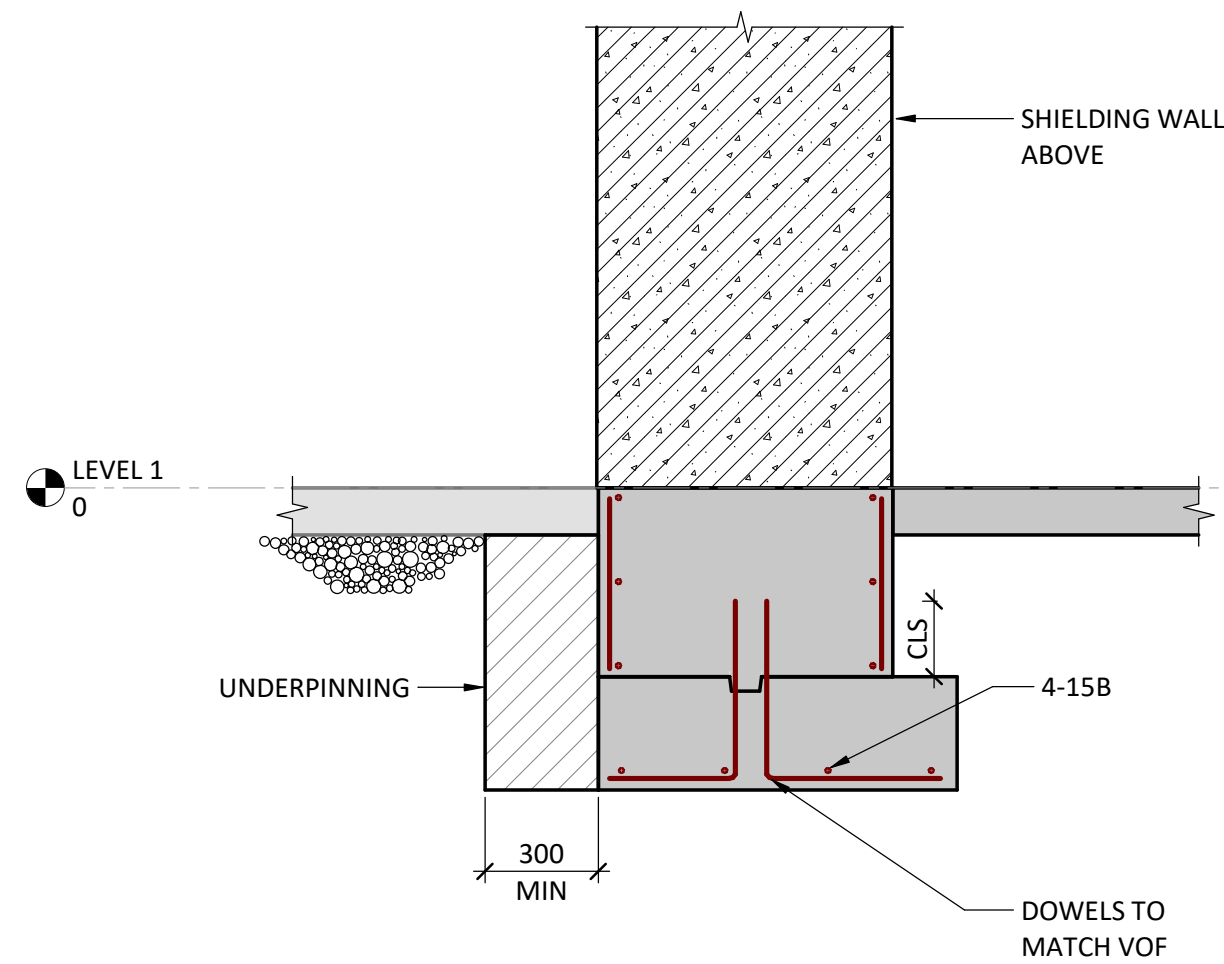
PROJECT:
THP CANCER CARE EQUIPMENT
2200 Eglinton Ave W,
Mississauga, ON L5M 2N1

TITLE:
ENLARGED FRAMING PLAN -
LEVEL 1 EAST

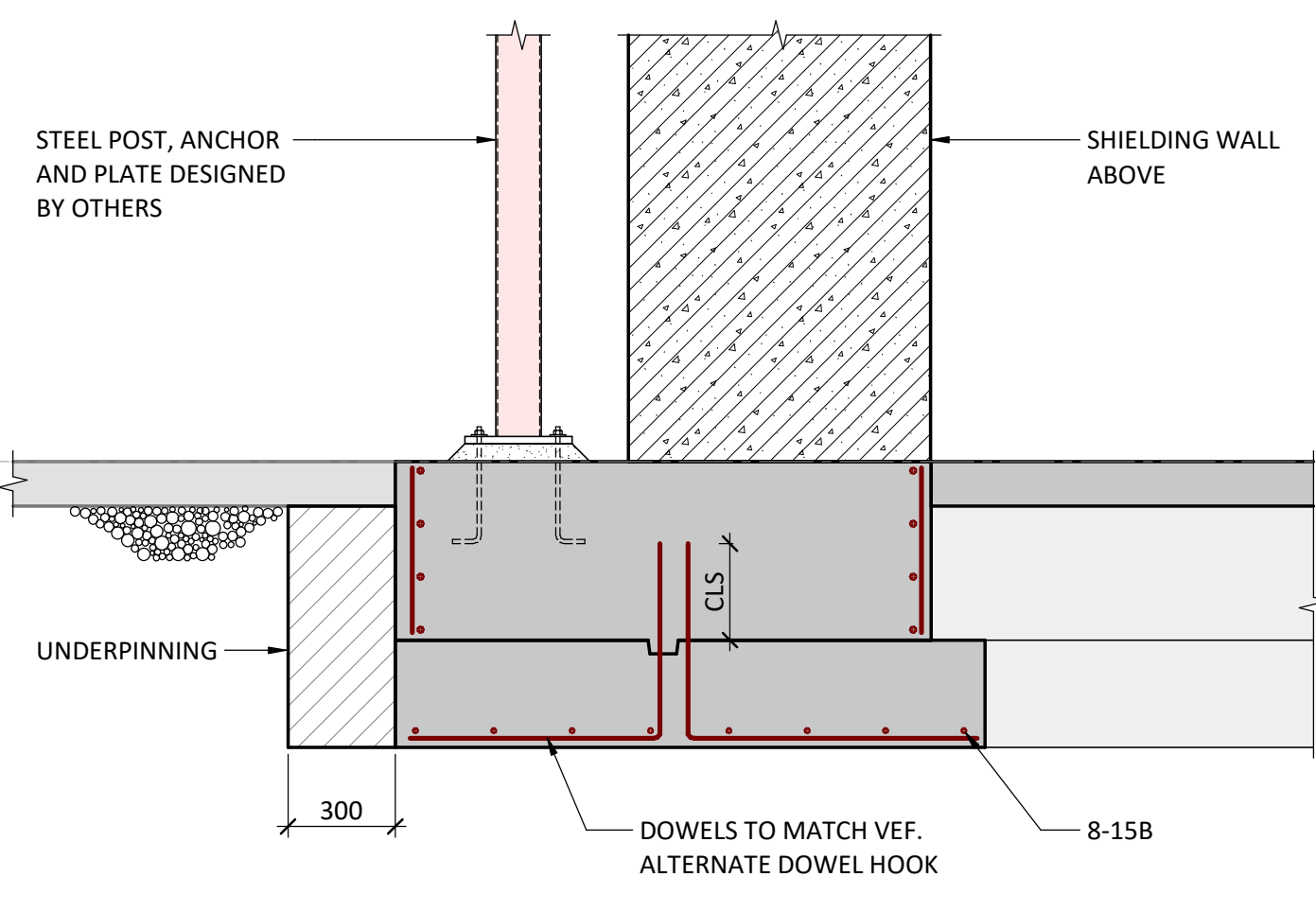
PROJECT NO:
EN023-01052
CHECKED:
BW

DRAWING NO:

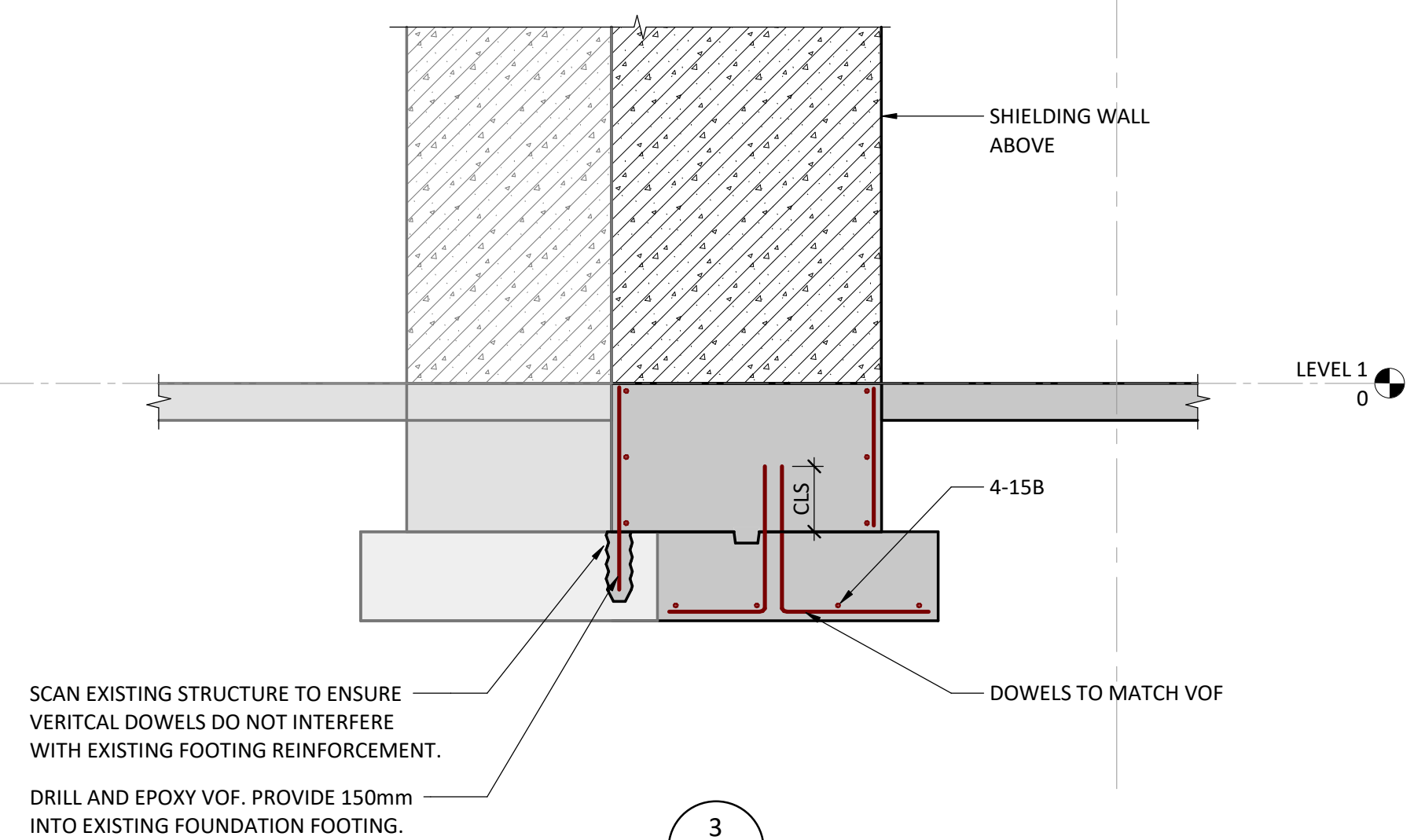
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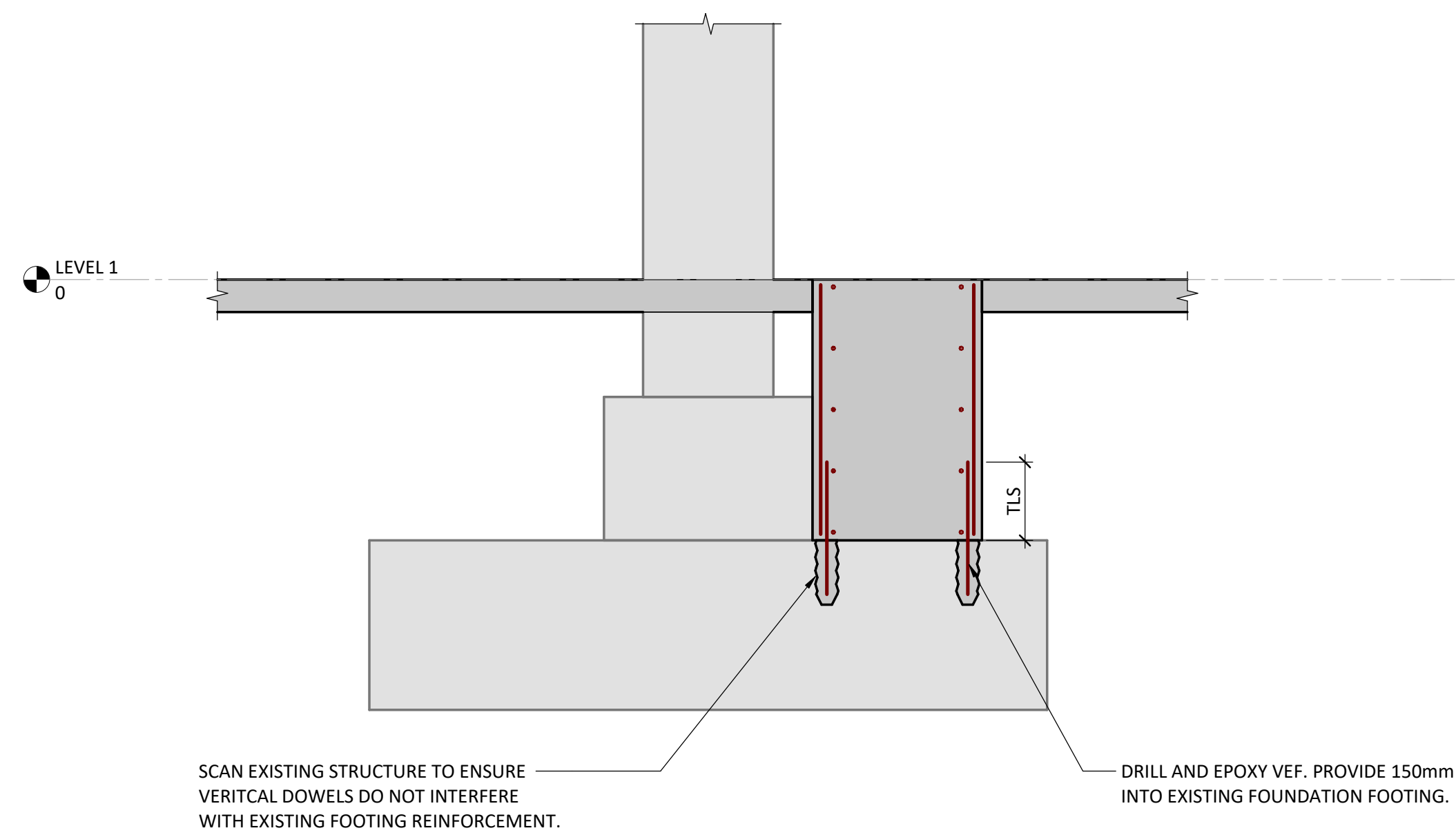
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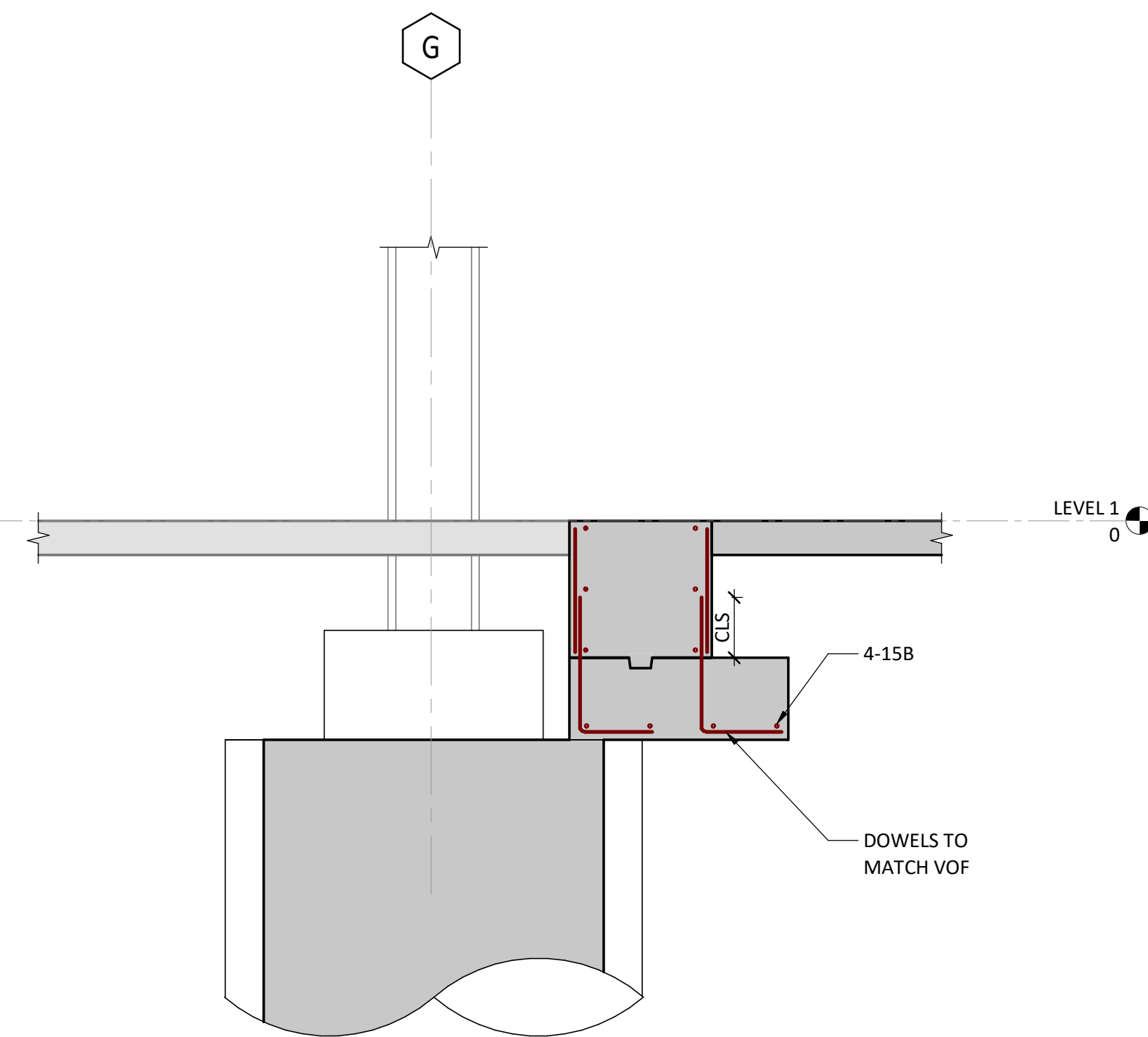
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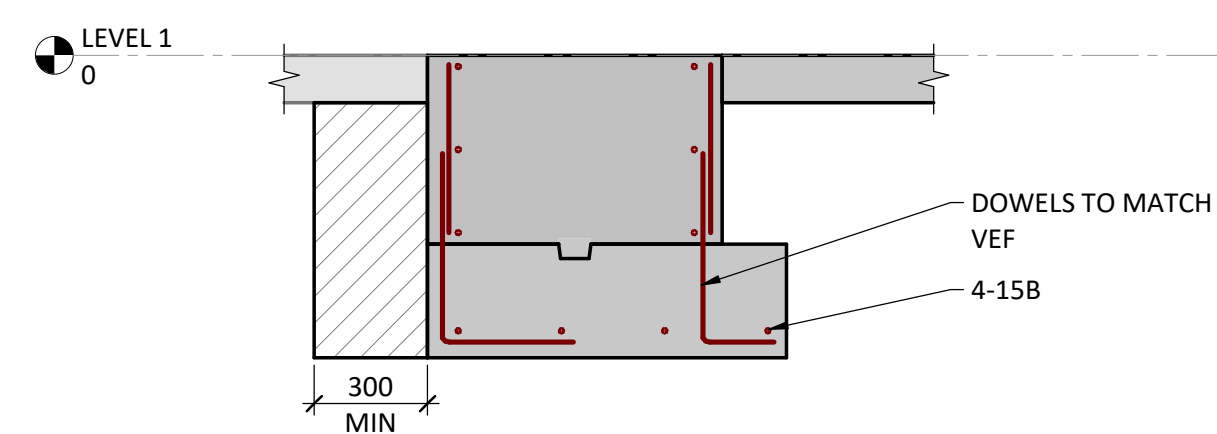
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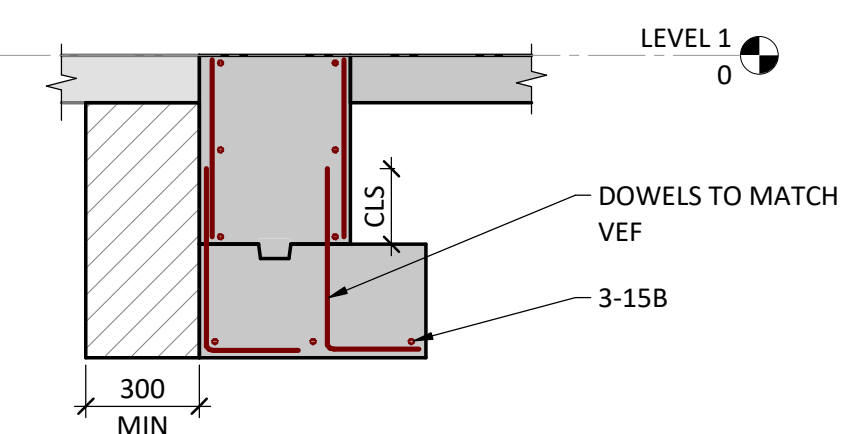
4
S400
1 : 20



5
S400
1 : 20



6
S400
1 : 20



7
S400
1 : 20

CLIENT:

CONSULTANT:

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SEAL:

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NO	DESCRIPTION	DATE
9	ISSUED FOR TENDER	2025/12/16
8	ISSUED FOR PERMIT	2025/11/28
7	MOH 2.3 RE-SUBMISSION	2025/06/13
6	MOH 2.3 SUBMISSION	2024/10/11
5	ISSUED FOR 100% CONSTRUCTION DOCUMENTS	2024/09/13
4	MOH 2.3 COSTING SUBMISSION	2024/06/17
3	ISSUED FOR PROGRESS	2024/06/04
2	ISSUED FOR PROGRESS	2024/05/03
1	MOH SUBMISSION	2023/10/18

PROJECT:
THP CANCER CARE EQUIPMENT
2200 Eglinton Ave W,
Mississauga, ON L5M 2N1

TITLE:
FOUNDATION SECTIONS & DETAILS

PROJECT NO:
EN023-01052
CHECKED:
BW

DRAWING NO:

S400